Alexis Fons

Construction Management

Advisor: Dr. Leicht



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9th Story Design Change

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Link Bridge Prefabrication

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Structural Lift System

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Building Details

Location | The Midwest

Size | 129,416 square feet

Height | 9 stories above grade

Cost | \$40.05 million

Duration | 17 months

June 2015 – October 2016





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Building Details

Location | The Midwest

Size | 129,416 square feet

Height | 9 stories above grade

Cost | \$40.05 million

Duration | 17 months

June 2015 – October 2016

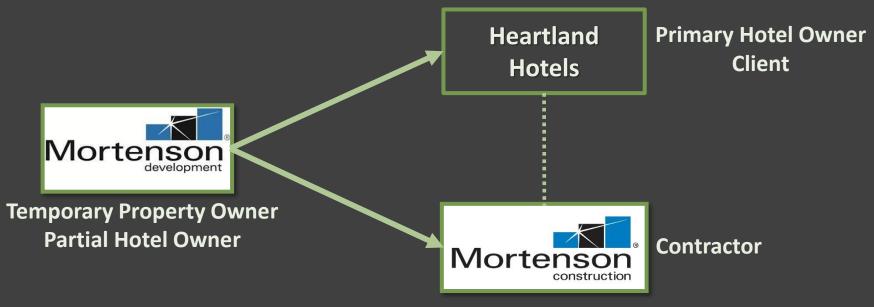




Project Delivery Method

Delivery Method | Design-Build (Turnkey)

Project Team:



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9th Story Design Change

Background Information

High-End Luxury Rooms
245 Standard Rooms

Opportunity Include Suites on 9th Story



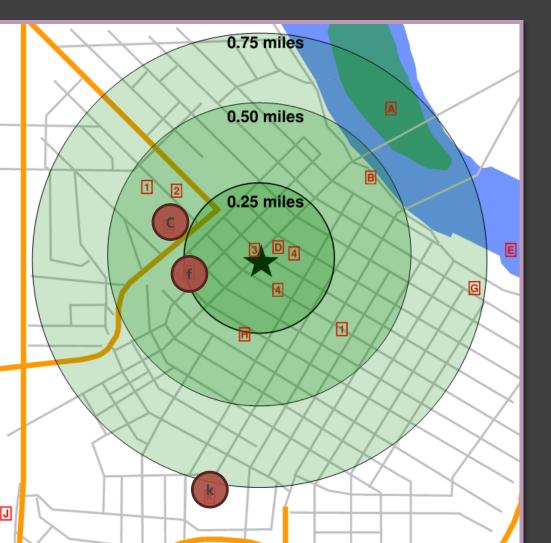
Points of Interest

Nearby Hotels

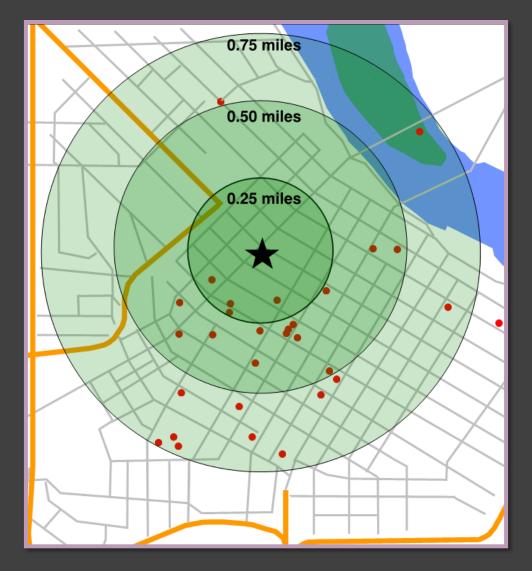
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Point Label	Points of Interest
Α	Historic Island
В	River Parkway
С	MLB Field
D	Library
E	Historical Bridge
F	Multi-purpose Arena
G	Historical Society museum
Н	Twelve-block Outdoor Mall
J	Art Center
K	Convention Center
Point Label	Public Transportation
1	Light Rail Station
2	Train Station
3	Bus Stop
4	Public Bike Sharing Stations



28 Hotels within 0.75 miles



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Nearby Hotel Prices

Suites

Standard

Rooms

Cost per Square Foot

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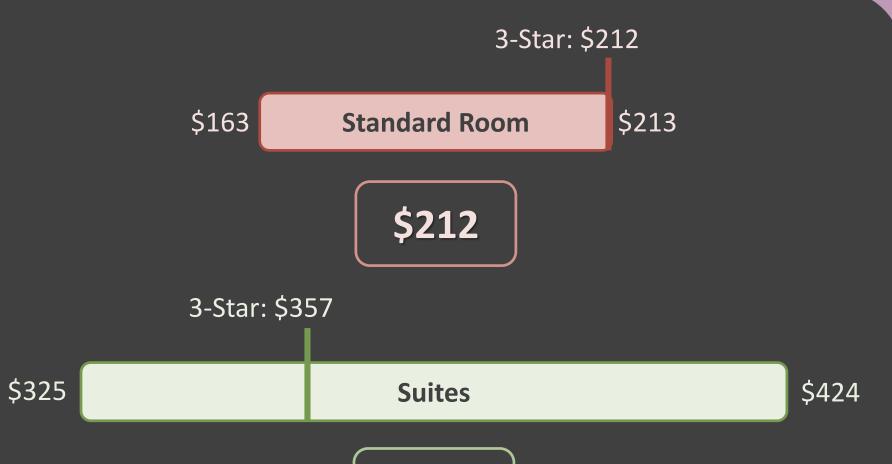
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\$360

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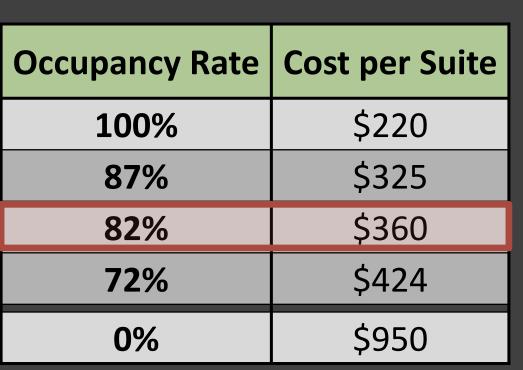
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5 Suite Study

10 Suite Study

ccupancy Rate	Cost per Suite
100%	\$220
62%	\$325
52%	\$360
42%	\$424
0%	\$950



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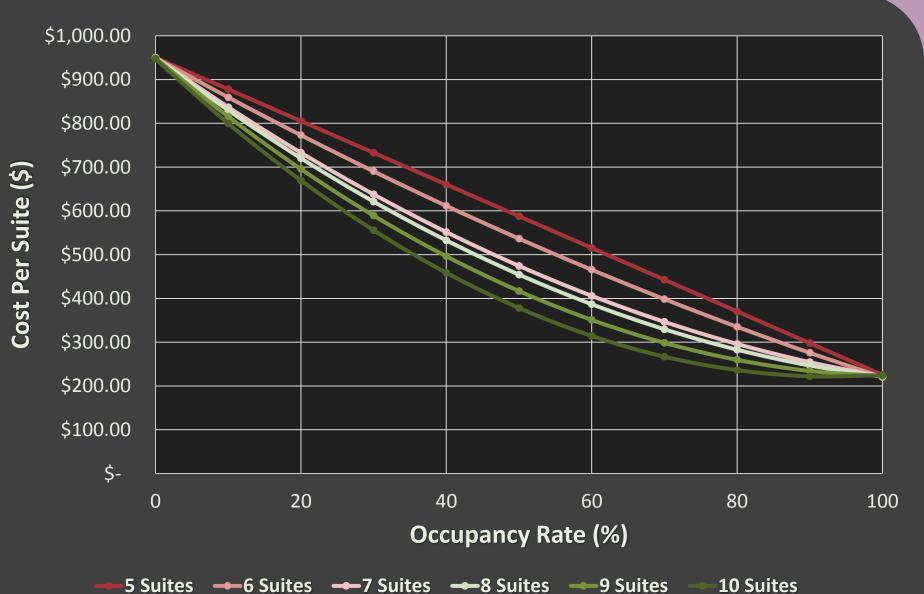


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Suite Profitability



Revenue of Suites

Suite Profitability

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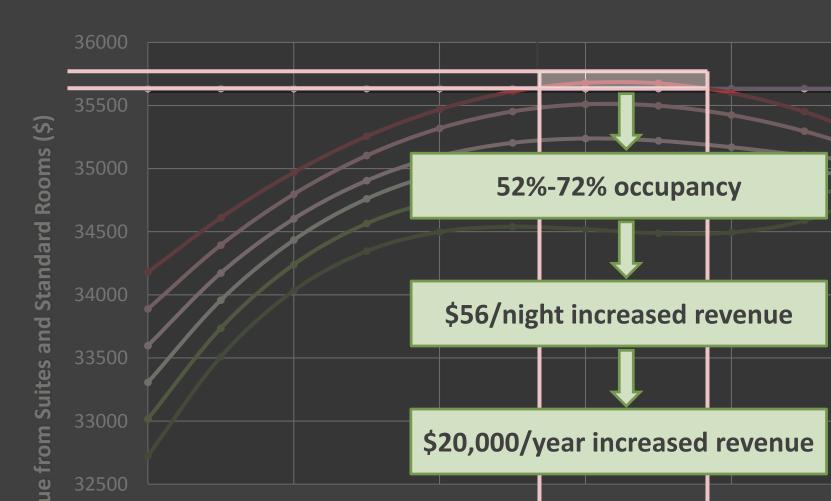
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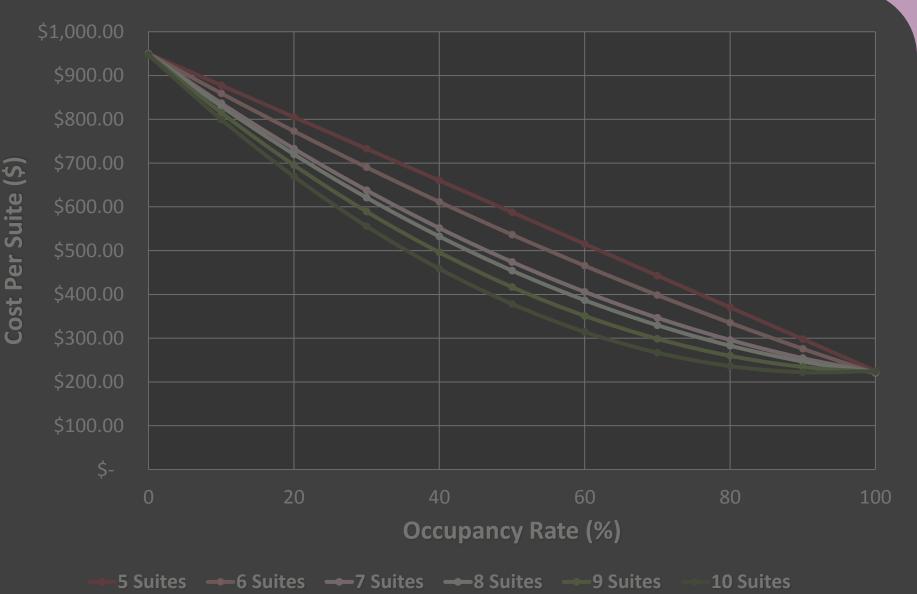
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Occupancy Rate (%)

Floor Plan Redesign

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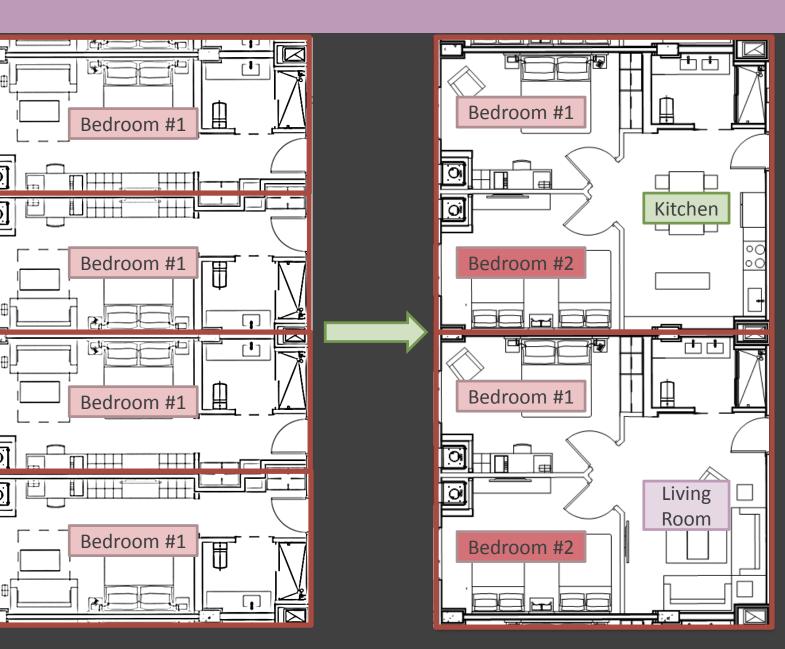


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Background Information **Locational Information** Occupancy Study Profitability Redesign **Cost Analysis**

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Finishes

Paint Flooring Wall Vinyl Tile

\$6,000

Furnishings

Standard Furniture Standard Fixtures Kitchen Equipment Living Room Furniture

Drywall

\$8,000

\$33,500 saved

Construction

Wall Studs Insulation

\$19,500



Schedule Analysis

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Background Information Locational Information Occupancy Study Profitability Redesign

Cost Analysis

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Lifecycle Analysis Final Recommendation Finishes

Paint Flooring Wall Vinyl Tile

90 man-hours

Furnishings

Standard Furniture Standard Fixtures Kitchen Equipment Living Room Furniture

Insulation

Drywall

10 man-hours

9% schedule decrease on the 9th floor

Construction

Wall Studs

190 man-hours

3 day reduction



Lifecycle Analysis

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Cost to Build \$40.05 million \$40.02 million

Revenue per Night \$35,630/night \$35,686/night

Cost-to-Run per Night \$25,755/night \$25,755/night

Capitalization Rate 9.00% 9.06%

Payback Period (Years)

11.11 years

11.04 years



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Analysis 1

9th Story Design Change



Recommended



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Link Bridge Prefabrication

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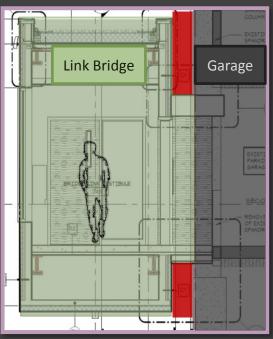
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Analysis 2

Link Bridge Prefabrication





A cross-section of the link bridge and the

Background Information

Current 62' Link Bridge **Design** Space Limitation

Opportunity Prefabrication – Modular Unit

Goals

Constructability

Bridge Components

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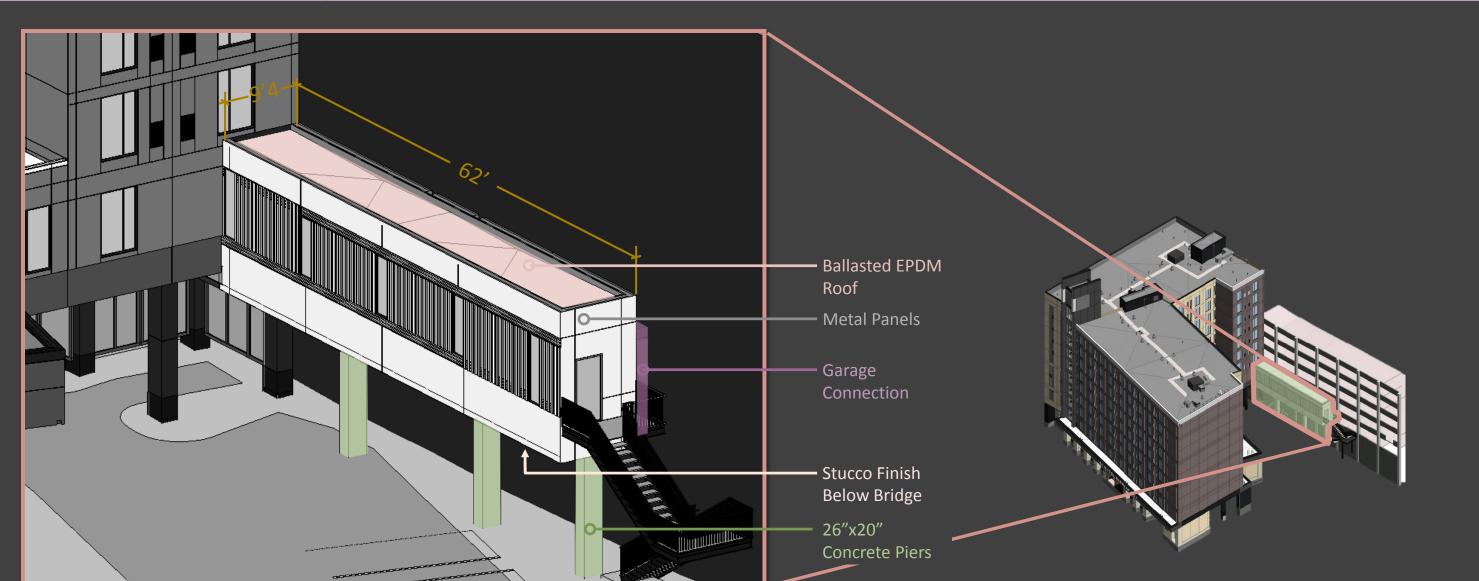
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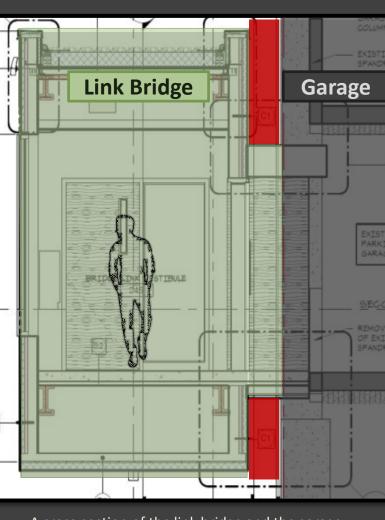


Link Bridge Prefabrication

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Crane Logistics
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A cross-section of the link bridge and the garage

Crane Logistics

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Crane Logistics

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Cost Analysis

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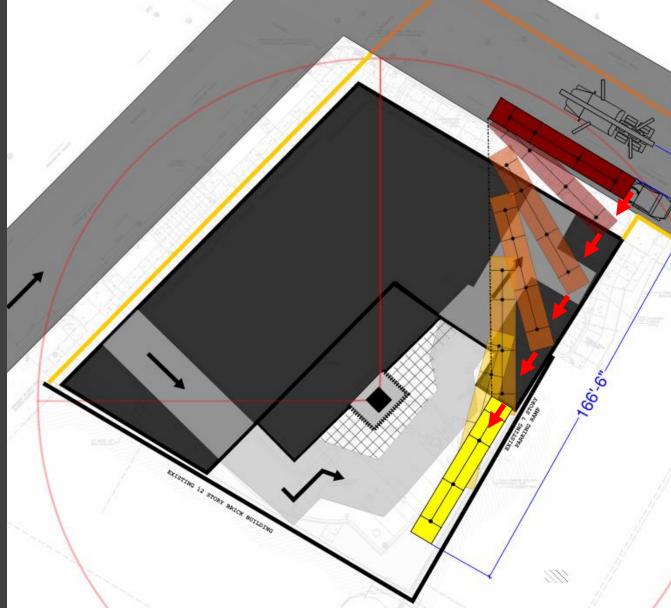
Bridge Weight | 19 tons

Tower Crane

Height/Reach | 213'
Max. Capacity | 7 tons

Hydraulic | Truck Crane |

Reach | 231'
Max. Capacity | 100 tons





Schedule Analysis

Productivity

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Final Recommendation

FRP 1st Level Columns

SOG

FRP 2nd Level Deck

Bridge

9% Increase in Productivity in Prefabrication Environment

On-Site Construction Critical Path Activities | 7 Days

7 Day Schedule Reduction

\$6,500 Labor Reduction



Cost Analysis

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Background Information

Crane Logistics

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Cost Analysis

Final Recommendation



Labor Reduction \$6,500

Truck Crane Usage \$1,400

Transportation Fee \$2,500

Prefabrication Overhead (3%) \$2,000

Temporary Pier \$800

Original Cost | \$79,000

Prefabrication Cost | \$76,000

\$26,500 Savings

↓

7 Day Schedule Reduction

\$27,000 Increased Profit



Final Recommendation

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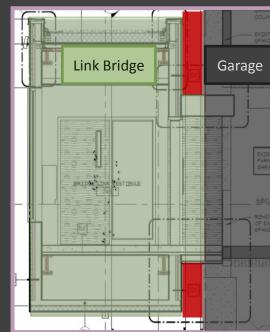


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Link Bridge Prefabrication





A cross-section
of the link
bridge and the



Recommended

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Structural Lift System

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Structural Breadth

Systems Design

Mechanical Breadth

Site Logistics

Schedule Analysis

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Analysis 3

Structural Lift System

Background Information

Stucco in winter weather Limited site space Renting multiple storage locations

Opportunity Upbrella Construction lift method

Constructability Innovation Schedule

Upbrella Construction

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Structural Lift System

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Final Recommendation





Advantages: Move in earlier

Pour Floor

Exterior Enclosure Below

Roof

Protection Shell

1 Level of Structural

Early landscaping

Increased safety

Warm environment (stucco)

No crane necessary

More room on site

Video courtesy of Upbrella Construction (https://www.youtube.com/watch?v=q9bKap4FdCc)

Current Construction Method

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Post-tensioned concrete floors

5 days/floor

1 level of shoring, 2 levels of re-shoring

Prefabricated column rebar

Tower crane

Structural Breadth

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Structural Lift System

Background Information

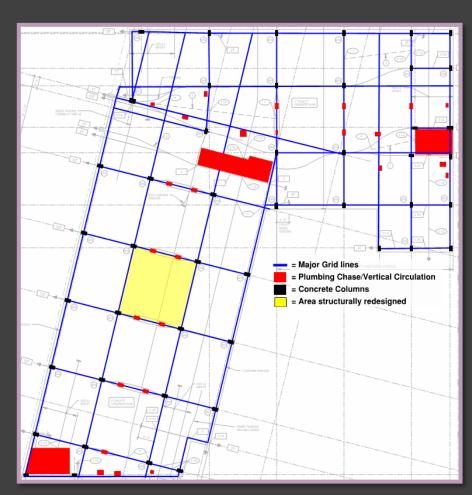
Structural Breadth

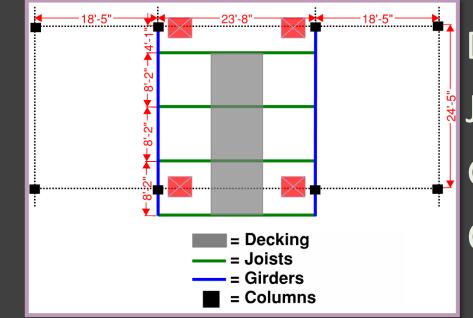
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PT Concrete (Current) on stressing ?

Delayed column strength





Loads:

Dead | 10 psf
Live | 40 psf
Deck + Concrete | 37 psf
Joists | 5 psf
Girders | 2 psf

Decking | 1.5VL20, t = 3.25" |
Joist | W12x26
Girder | W14x30

Columns | W10x49 (floors 1-9)
W10x33 (roof)



7' taller building to maintain floor-to-ceiling height

Hydraulic Cylinders

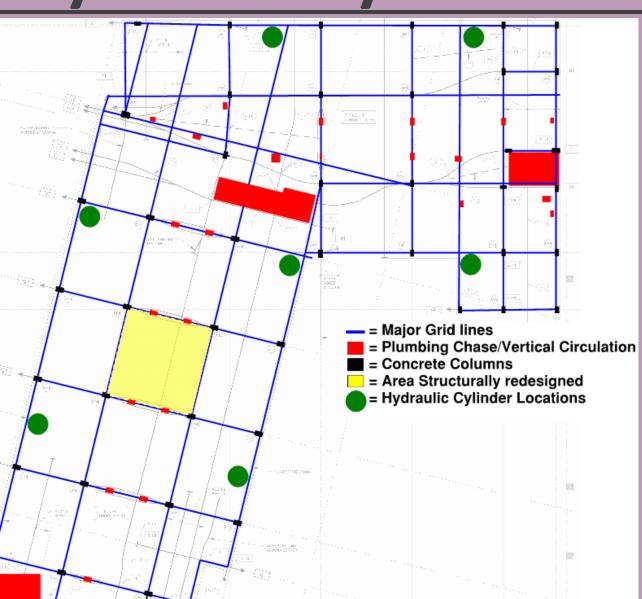
Protection Shell

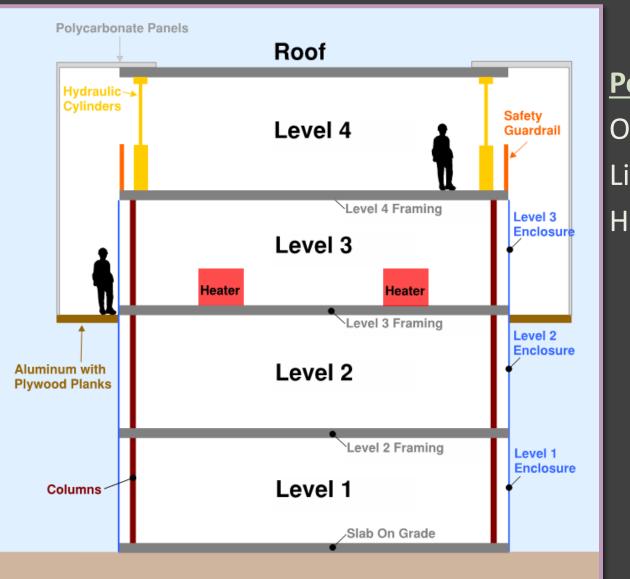
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Structural Lift System

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Polycarbonate Panels:

OSHA

Light Transmission

High R-Value

Mechanical Breadth

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Concrete Requirements: 55°F Original Method

Material	Square Footage	R-Value	U-Value	Temperature Change	Total Btu/hr	
Tarp	6,240	0.071	14.11	28.5	2,509,322	
ormwork (3/4" plywood)	14,596	0.94	1.06	28.5	442,538	
oncrete (NW 6.57" thick)	14,596	0.47	2.13	28.5	885,077	
Total Btu/hr for total construction: 3,836,937						
This is equivalent to 4 heaters						

Stucco Requirements: 40°F Original Method

Material	Square Footage	R-Value	U-Value	Temperature Change	Total Btu/hr
arp (between scaffolding and building)	6,240	0.071	14.11	10.4	915,683
Tarp (exterior)	6,600	0.071	14.11	10.4	968,510
Scaffolding Wood Planks (1.5" thick)	6,438	1.88	0.53	10.4	35,614
	Tot	al Btu/hr	for total	construction:	1,919,807
This is equivalent to 2 heaters					

Mechanical Breadth

Systems Comparison

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Constant Heating: 55°F Proposed Method

Material	Square Footage	R-Value	U-Value	Temperature Change	Total Btu/hr	
Polycarbonate Panels	17,079	2.000	0.50	27.5	234,836	
Plywood Planks (1.5")	3,219	1.880	0.53	27.5	47,086	
Concrete (3.25")	14,596	0.260	3.85	27.5	1,543,808	
Roof	14,596	24.000	0.04	27.5	16,725	
Metal Deck	14,596	0.000	N/A	27.5	-	
Solar Heat Gain Coefficient	N/A	N/A	N/A	N/A	(63,749)	
Total Btu/hr for total construction: 1,778,70						
This is equivalent to 2 heaters						

Original Method

Concrete at 55°F Stucco at 40°F

140 days \$21,100 of heat

Structural Lift Method Both systems at 55°F

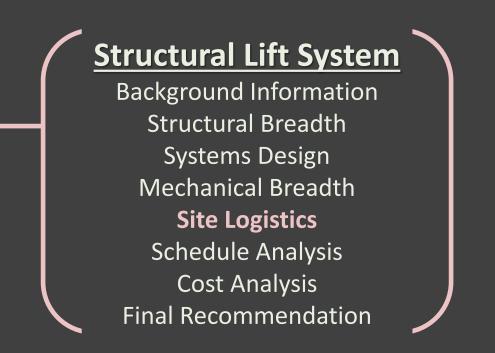
136 days \$19,500 of heat



Site Plan: Original

Site Plan: Proposed

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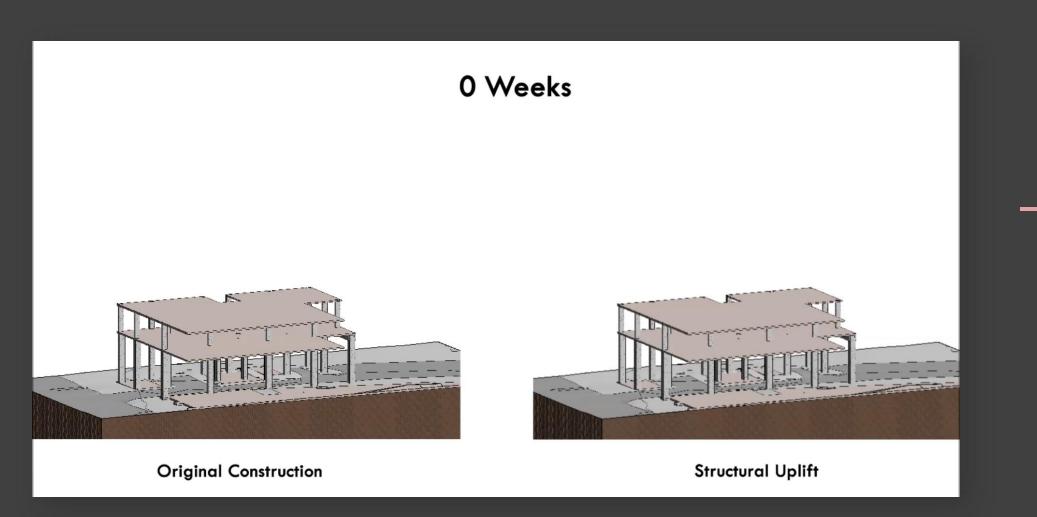


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Original Method:

291 days

5 days/floor of structure

Proposed Method:

258 days – 7 weeks shorter

11 days per floor overall

9% productivity increase

Interior Activities – additional overlap

Cost Analysis

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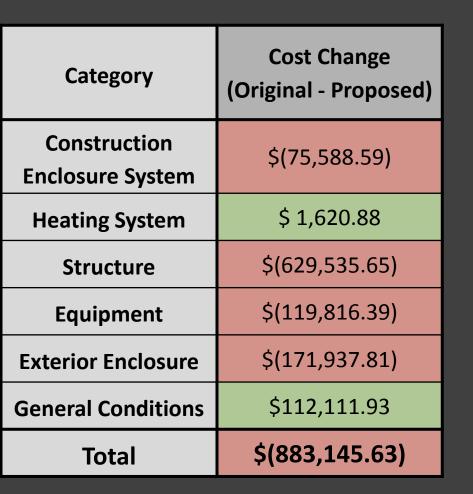
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\$883,000 cost increase

33 Day Schedule Reduction

\$170,000 Increased Profit



Final Recommendation

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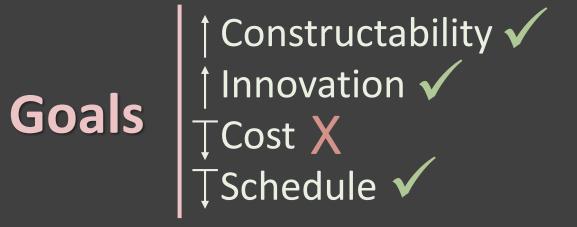
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Structural Lift System



Not Recommended

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Field Labor Experience

Background Information

Field Labor Office Work

Superintendent

What benefits does field labor experience provide for a career as a superintendent?

How does field labor impact the role and success of a superintendent?

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Field Labor Experience

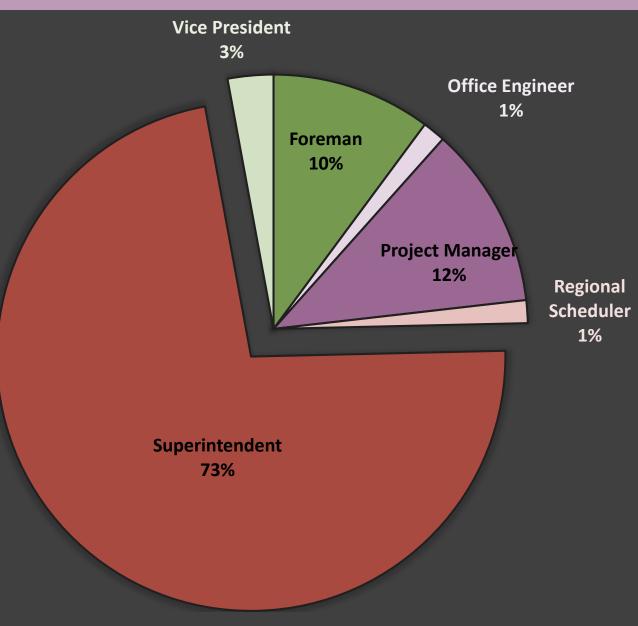
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Current Position



Level of Education

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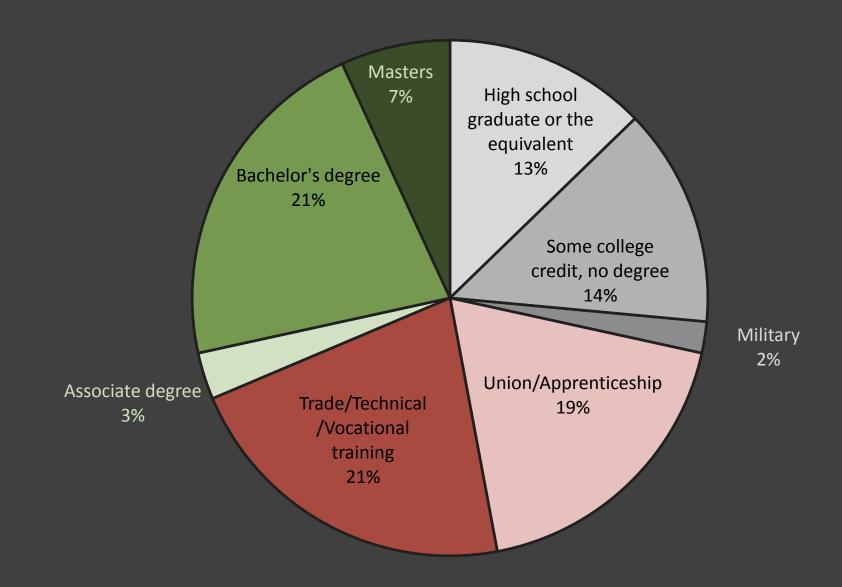


Field Labor Experience

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Level of Education

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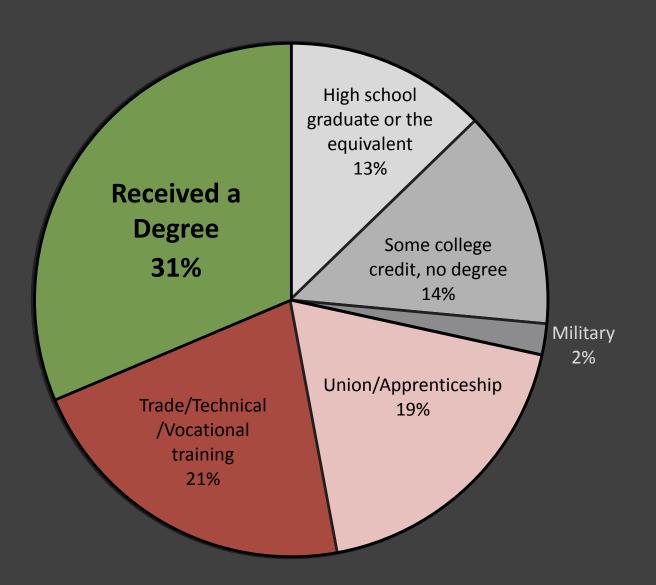
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Current Position

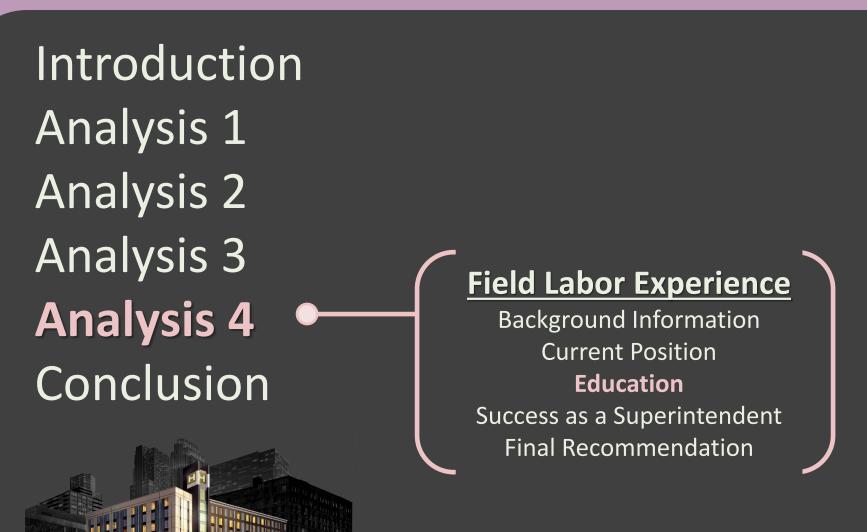
Education

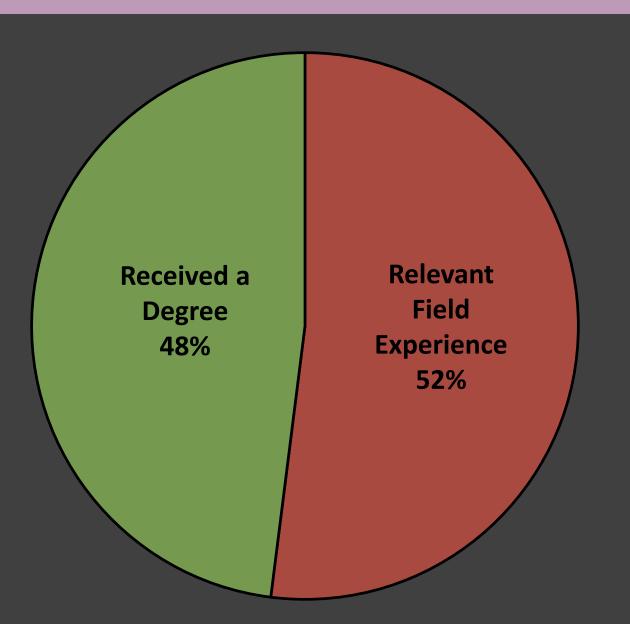
Success as a Superintendent Final Recommendation



31% of all respondents received a degree

Superintendent Level of Education





48% of superintendents received a degree

Construction Management and Civil Engineering

52% of superintendents went into the field

Carpentry (48%)

Success as a Superintendent



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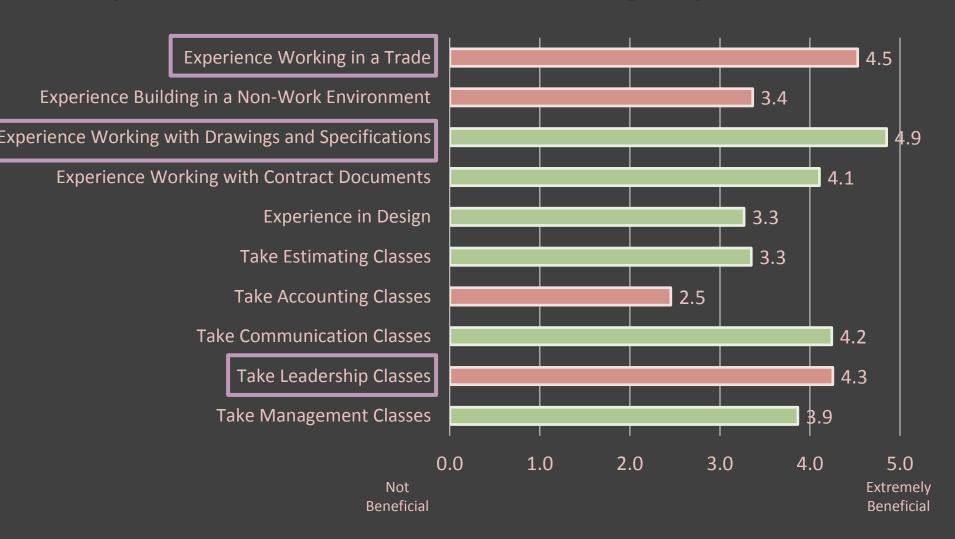
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Experiences which are beneficial to becoming a superintendent



Experience Working with Drawings and Specifications

4.9/5

Experience Working in a

4.5/5

Taking Leadership Classes

4.3/5

Success as a Superintendent

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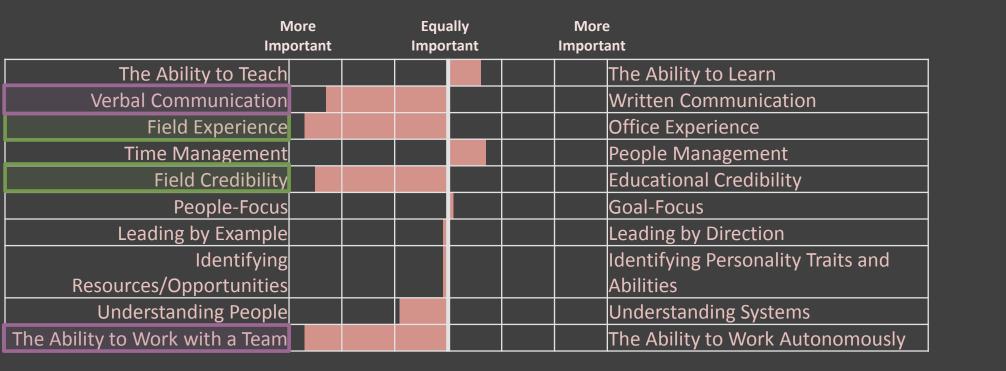
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Field Labor Experience

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Importance of skills/attributes for becoming a superintendent



Office vs. Field Experience

Importance of Relationships

Office vs. Field Experience

Field Experience
Field Credibility

Importance of Relationships

Verbal Communication
The Ability to Work with a
Team

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Field Labor Experience

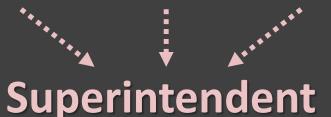
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Field Labor Experience

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Field Laborld and Office Woffike Work



According to current superintendents, field labor experience is more valuable for a career as a superintendent.

Those receiving a higher education – go out and build.

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Analysis 1

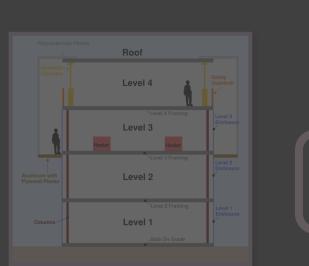
9th Story Design Change

[Recommended]

Analysis 2

Link Bridge Prefabrication





Analysis 3

Structural Lift System





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Analysis 1

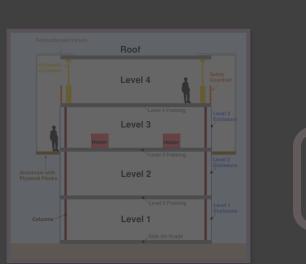
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Analysis 3







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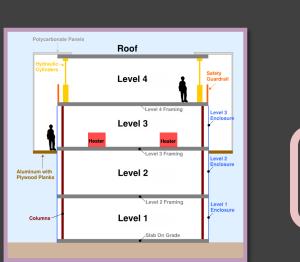
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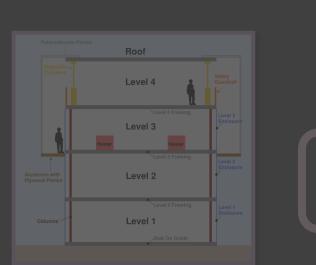
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Field Labor Experience

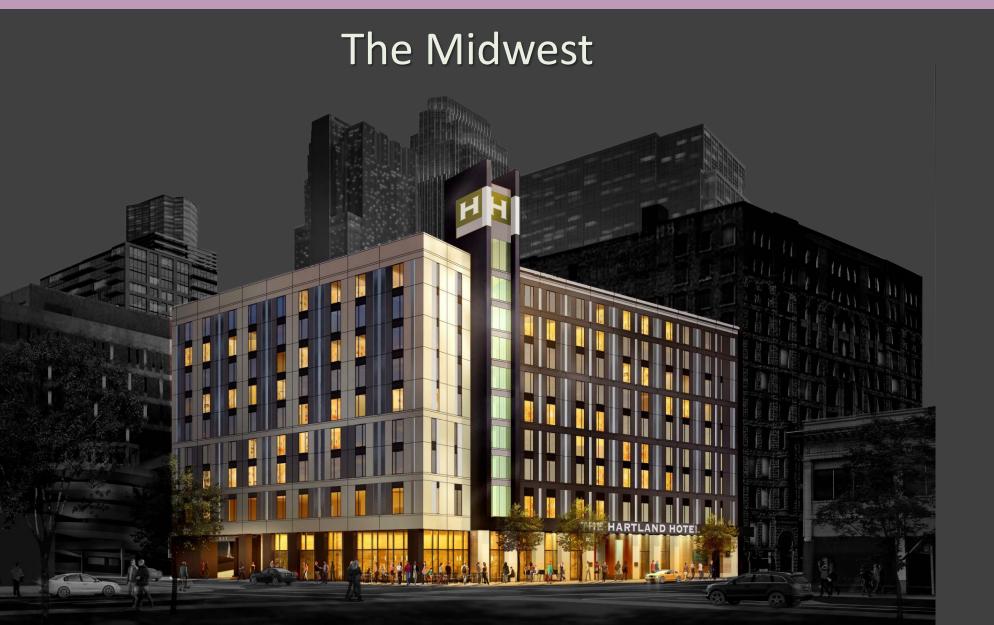
Extremely Beneficial





Thank you

Jesus
PSU AE Faculty
Mortenson Construction
DPR Construction
Family and Friends



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9th Story Design Change Appendix

Mortenson Development Inc.

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Partial Owner

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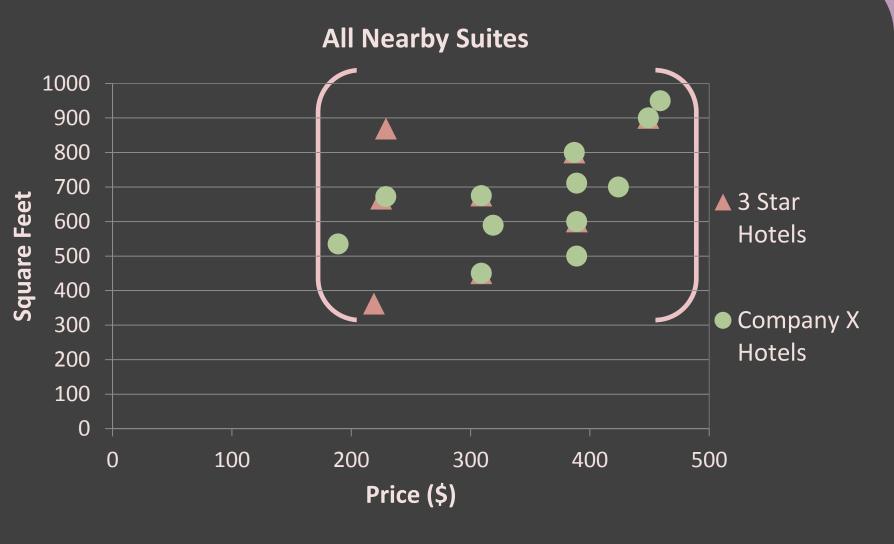
Analysis 4

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Nearby Hotel Prices





5 Suite Study

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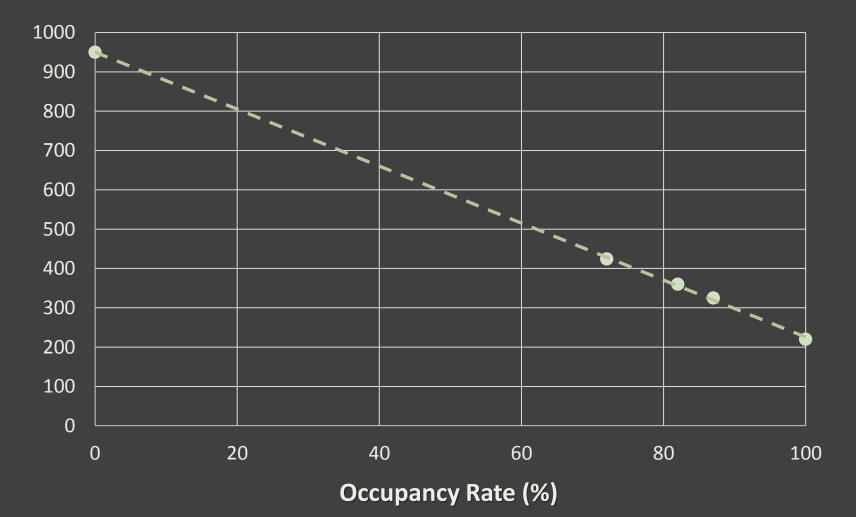
20	100%	
.ZU	 TUU /0	

\$950 ---> 0%

ost per	Occupancy
Suite	Rate
\$220	100%
\$325	87%
\$360	82%
\$424	72%
\$950	0%

Cost per Suite (\$)

5 Suites



5 Suite Study

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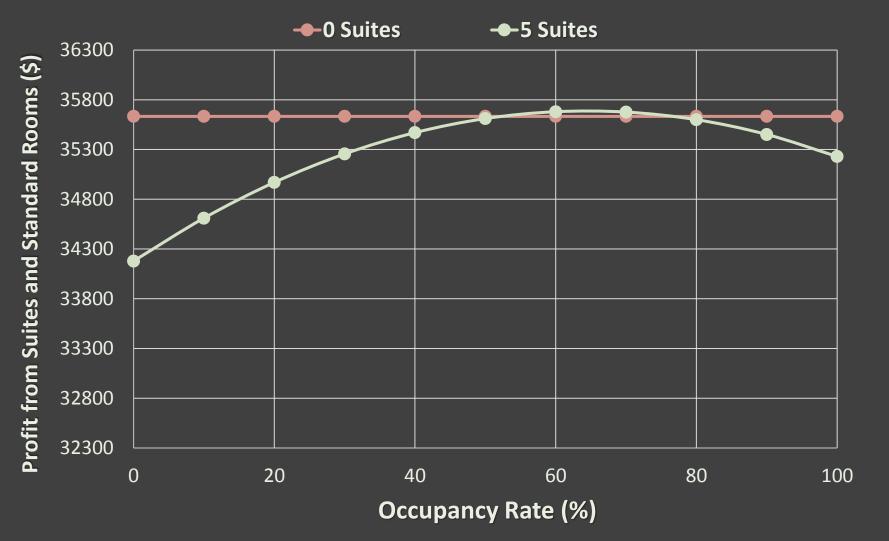
20	 100%
ZU	 TOO /O

\$360 ---> 82%

\$950 ---> 0%

Cost per	Occupancy
Suite	Rate
\$220	100%
\$325	87%
\$360	82%
\$424	72%
\$950	0%

Revenue of 5 Suites



10 Suite Study

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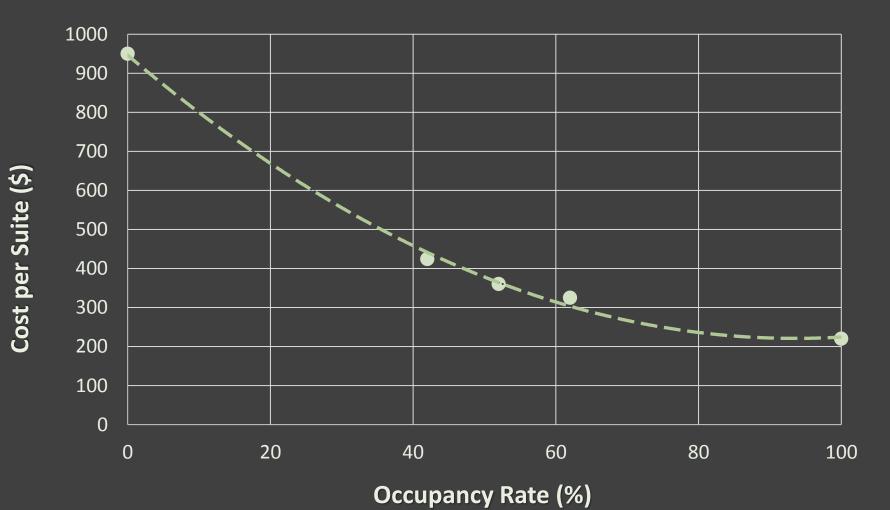
20	100%
ZU	TOO /0

\$360 ---> 52%

\$950 ---> 0%

ost per	Occupancy
Suite	Rate
\$220	100%
\$325	62%
\$360	52%
\$424	42%
\$950	0%

10 Suites



10 Suite Study

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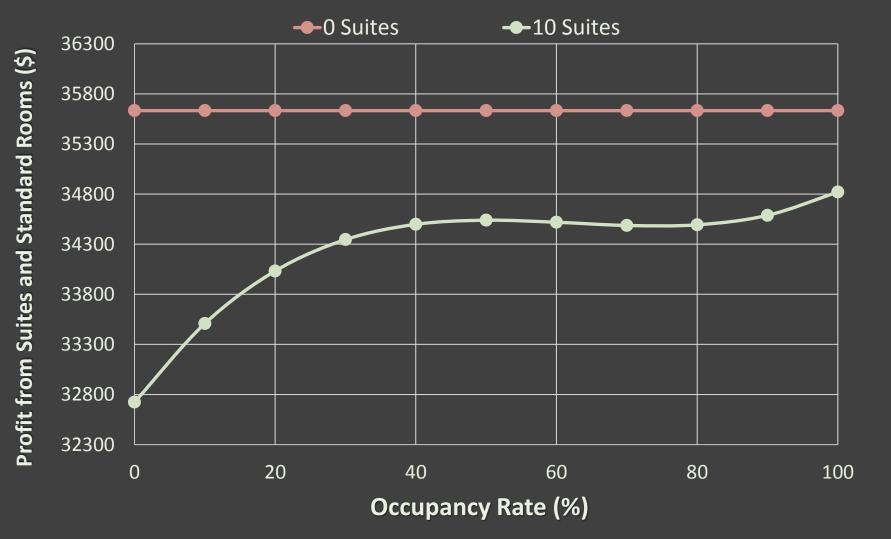


20	1000/
ZU>	TOO 20

\$950 ---> 0%

Cost per	Occupancy
Suite	Rate
\$220	100%
\$325	62%
\$360	52%
\$424	42%
\$950	0%

Revenue of 10 Suites





Advantages and Disadvantages

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On-Site Construction



Prefabrication

Panels

Small delivery truck

Lighter crane pick 🗸

Quality control 🗸

Limited access X

Integrity of seams?

Attachment methods ?

No interior finishes?

3D Module

Complete entire unit 🗸

Design already created 🗸

Minimal site construction

Best quality control

Larger pick ?

Larger truck ?

Connections

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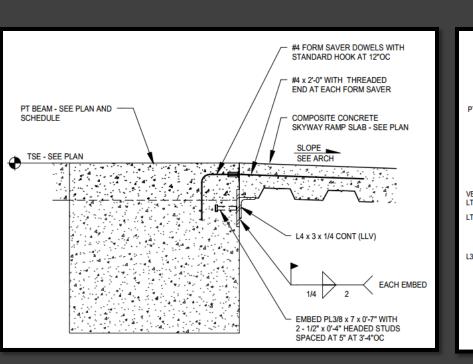
Analysis 2

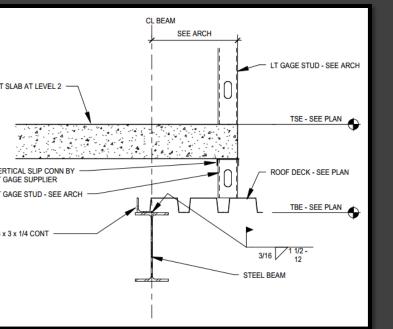
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Heartland Hotel Budget

ACTUAL BUDGET	COST
ndations	\$ 713,268.10
erstructure	\$ 4,474,147.48
rior Enclosure	\$ 2,341,547.31
fing	\$ 400,820.83
rior Construction	\$ 8,111,096.92
rior Finishes	\$ 3,231,833.54
veying & Chutes	\$ 807,362.00
hanical	\$ 5,176,729.00
Protection	\$ 296,887.50
trical	\$ 2,081,009.59
Voltage	\$ 757,500.00
pment	\$ 25,500.00
ding Demolition	\$ 7,167.73
Preparation	\$ 466,617.23
Improvements	\$ 147,464.00
Electrical Utilities	\$ 81,950.00
Services	\$ 541,870.30
niture, Fixtures & Equipment (FF&E)	\$ 3,411,740.28
rating Supplies & Equipment (OS&E)	\$ 830,015.03
eral Conditions/Staff	\$ 1,732,638.90
nits & Street/Sidewalk Closure Fees	\$ 573,166.00
d Party Inspections/Testing	\$ 160,000.00
gn Fees	\$ 950,000.00
rance	\$ 351,558.00
gn Builders Fee	\$ 1,071,898.00
mating & Construction Contingency	\$ 1,306,212.00
al:	\$ 40,049,999.74

Structural Lift Method Appendix

Upbrella Construction Breakdown

Schedule Break Down

Introduction

Analysis 1

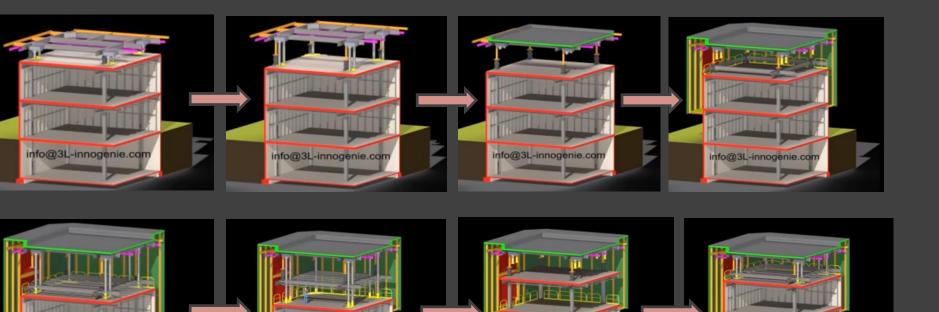
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Task Name	Duration	
Floor Structure		
Construct Floor Framing (Joists and Girders)	5 days	
Lift and Install Columns	1 day	
Lower System onto Columns	1 day	
Finalize Structure (Stairs, Elevator Framing, Rebar, Inspection)		
Pour Floor Concrete	1 day	
Wall Enclosure Systems		
Exterior Infill Framing	8 days	
Install Stucco	9 days	
Install Metal Panels	9 days	
Install Glazing Systems	9 days	

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Cost Breakdown

	Category	Material	Description	Amount	Floor Multiplier	Unit	R.S. Means page	Labor- Hour	Material	Ī	Labor	Equipment	Total	Areas Totaled	Man-Hours
	Construction	Polycarbonate Panels	Walls and ceiling	17079.0	1	sf		0.107	\$ 1.2	26	\$ 5.64		\$ 117,722.1	3	1,827.45
	Enclosure	7' Aluminum with plywood planks	Floor	88.6	1	ea.		0.33	\$ 115.6	9	\$ -		\$ 10,250.1	\$ 224,233.26	29.24
	System	Steel Columns (W10x33)		2310.0	1	lf	310	0.093	\$ 32.1	9	\$ 5.61	\$ 2.78	\$ 93,747.4	7 \$ 224,233.20	214.83
L	System	Guardrails	Portable metal with base pads	624.0	1	lf		0.027	\$ 2.5	57	\$ 1.46		\$ 2,513.5	2	16.85
	Heating	Heaters	136 days of heat	4.5	1	month		0	\$ -	_	\$ -	\$ 1,300.00		S 19 592 80	-
ह्र	System	Propane for Heaters	8.25 GPH at 81%	7270.6	1	Gallons		0	\$ 1.8	_	\$ -	\$ -	\$ 13,699.4	/	-
ŧ	L	1.5VL20 decking	22 ga., 2" galvanized	14596.0	8	sf	168	0.009	\$ 2.4	_	\$ 0.55		\$ 352,200.3		1,050.91
ž	L	Concrete topping (3.25")	Lightweight	14596.0	8	sf	97	0.022	\$ 1.2	_	\$ 1.06	\$ 0.27	,		2,568.90
8		WWF (6x6, W1.4xW1.4)		146.0	8	csf	95	0.457	\$ 14.5	_	\$ 27.58		\$ 49,234.0		533.63
ĕ	Structure	Joists (W12x16)		1793.0	8	lf ''	158	0.064	\$ 23.6	_	\$ 3.82	\$ 1.74	+,	─ 5 1 837 730 07	918.02
ę.	H	Girders (W14x30)		965.0 517.0	8	lf lf	158 158	0.062	\$ 43.7 \$ 32.1	_	\$ 3.74 \$ 5.61	\$ 1.70 \$ 2.78			478.64
ε		Columns (W10x33 - roof)		317.0	-	IT	158	0.093	\$ 32.1	7	\$ 5.61	\$ 2.78	\$ 20,981.5	8	48.08
yste		Columns (W10x49 - all other levels)		485.7	8	If	158	0.102	\$ 71.9	3	\$ 6.11	\$ 2.55	\$ 313,155.6	6	396.33
# S		Double Girder Bridge Crane	2 Girder, 50' span, 3 ton	2.0	1	ea.	1200	72	\$ 46,753.0	_	\$ 4,766.03	\$ 355.00	\$ 103,748.0	5	144.00
ᆵ	L	Hydraulic Cylinders		7.0	1	ea.		0	\$12,492.0	_	\$ -		\$ 87,444.0	_	-
= 1	Equipment	Material Hoist	2 for 5 months	10.0	1	month	1335	0	\$ 2,892.2	25	\$ -	\$ -	\$ 28,922.5	0 \$ 264,680.39	-
Structural Uplift System (Proposed Method)	Equipment	Mast-Climbing Platform	2 for 4 months, 50' wide, less than 100' tall, rent	8.0	1	mast*month	21	0	\$ 3,118.6	60	\$ -	\$ -	\$ 24,948.8		-
ş		Street Cranes	Truck mounted, 150 tons, 18' radius	10.0	1	days		0	\$ 1,961.7	70	\$ -	\$ -	\$ 19,617.0	0	-
	Façade	Additional Exterior Enclosure Installation	7' taller building w/624' perimeter; added price is a multiple of the original exterior enclosure cost	4368.0	1	sf		0.28	\$ -		\$ -	\$ -	\$ 171,937.8	1 \$ 171,937.81	1,223.04
•	Profit	Profit from Opening	Opens 1.5 months earlier	0.0	0			0	\$ -		\$ -	\$ -	\$ (170,169.0	0) \$ (170,169.00) -
ı												Tota	l: \$ 2,345,006.1	9 \$ 2,345,006.19	9,449.91
	Construction	Scaffolding Framing Erection	6-12 stories, 6'4x5	32.9	10.6	100sf	19	3.6	\$ 35.2	21	\$ 196.28		\$ 80,728.5	3	1,255.46
	Enclosure	Scaffolding planks 2"x10"x16'	Average cost of below and above 50'	241.0	10.6	ea.	20	0.333		_	\$ 18.34		\$ 62,282.9		850.68
	System	Scaffolding Tarp	Polyethylene sheet	59.7	7	100sf	23	0.216	\$ 4.0	00	\$ 9.47		\$ 5,633.2		90.29
8	Heating	Heaters	140 days of heat at 87%	4.7	1	month		0	\$ -	_	\$ -	\$ 1,300.00		7	
ま	System	Propane for Heaters	8.25 GPH at 87%	8038.8	1	Gallons		0	\$ 1.8	88	\$ -	\$ -	\$ 15,147.0	\$ 21,213.68	-
Σ		Concrete Slab	6" elevated	14596.0	8	sf	98	0.022	\$ 2.0)4	\$ 1.06	\$ 0.27	\$ 393,192.8	9	2,568.90
Original Construction Method	ſ	PT Formwork	Multiple by number of forms - 4 uses, so 2x floor	14596.0	3	sf	76	0.086	\$ 1.1	9	\$ 4.59		\$ 253,061.8	О	3,765.77
ast.	I	PT Tendons		9075.0	8	lb.	95	0.027	\$ 0.6	51	\$ 1.63	\$ 0.03	\$ 165,114.1	8	1,960.20
ខ	Structure	Slab Rebar	Elevated slabs, #4-#7	860.0	8	lb.	93	0.006	\$ 0.5	50	\$ 0.34	\$ -	\$ 5,770.3	\$ 1,205,195.30	41.28
<u> </u>		Column Concrete	16x16, less than 2% reinf, 9.57'	30.5	9	су	97	0	\$ 283.6	69	\$ 677.44	\$ 46.00	\$ 276,818.9	3	-
g.		Column Rebar	#3-7 Column Rebar	19936.5	1	lb.		0.011	\$ 0.5	0	\$ 0.65	\$ -	\$ 22,936.1	0	219.30
ō		Column Rebar	#8=18 Column Rebar	95703.7	1	lb.		0.007	\$ 0.5	0	\$ 0.42	\$ -	\$ 88,301.0	1	669.93
- [Equipment	Tower Crane		7.5	1	month	1334	0	\$ 17,001.4	10	\$ -	\$ -	\$ 127,510.5	0 \$ 144,864.00	-
L	Edorbinem	Material Hoist	6 months	6.0	1	month	1335	0	\$ 2,892.2	25	\$ -	\$ -	\$ 17,353.5	0 \$ 144,884.00	-
[Tota	l: \$1,519,917.6	4 \$ 1,519,917.64	11,421.81
									Compara	tive	Total (Propo	sed - Original): \$ 825,088.5	5 \$ 825,088.55	(1,971.89)

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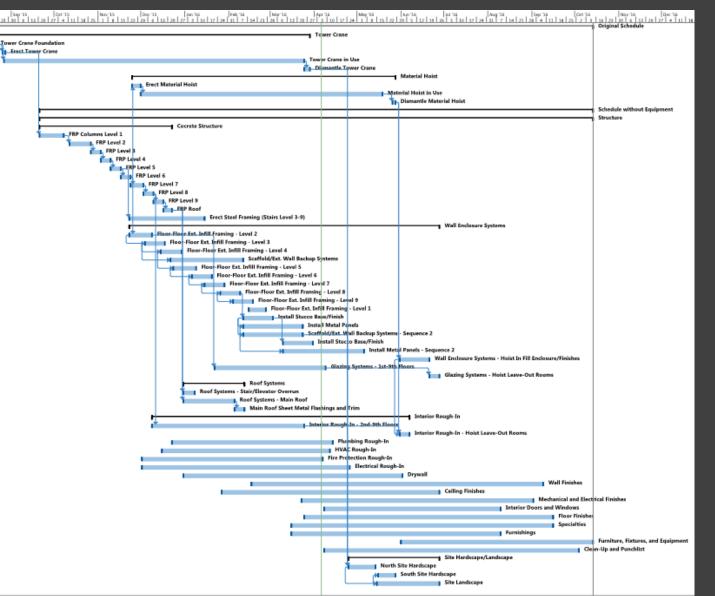
Analysis 3

Analysis 4

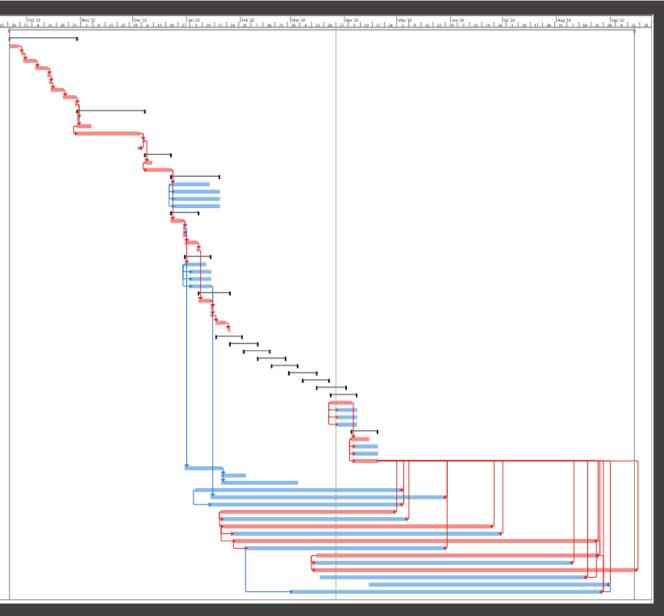
Conclusion



Original Schedule



Proposed Schedule



Structural Breadth Calculations

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Decking

Requirements

- Composite decking because:
 - Thinner
 - More economic use of materials
- 3-1/4" topping for fireproofing for lightweight concrete
- 3-spans
- 8'2 spacing
- Unshored

Loads taken from the structural drawings:

- Live Load 40psf for rooms and corridors above the 2nd floor
- Wind Load N/A
- Dead Load assumed 10pst

 $Total\ Load = Dead + Live$

$$50psf = 10 + 40$$

Decking chosen from Vulcraft Steel Roof and Floor Deck catalog:

1.5VL20 with lightweight concrete

4.75" deep

t=3.25" (for fireproofing)

37psf = concrete + deck self-weight

Recommended Welded Wire Fabric: 6x6 - W1.4xW1.4

Maximum superimposed load with a 8'6 clearspan: 200psf ✓

- Joists = Assumed 5psf
- Superimposed dead load = 10psf
- Live load = 40psf
 - Live Load reduction: $K_{II}A_{T} = 8'2*2*23'8 = 387 < 400 not reducible$

Load Combinations

- Dead: 37+5+10=52psf
- Live: 40psf

1.4D = 75.6psf

1.2D+1.6L = 126.4psf - controls

Check Decking:

126.4psf < 200psf limit ✓

Weight:
$$W_u = 126.4psf * \frac{8'2}{1000\frac{lb}{lin}} = 1.03 \ klf$$

$$M_U = \frac{wl^2}{8} = \frac{(1.03klf)(23'8)^2}{8} = 72.11ft \cdot kips \rightarrow W12x16^2$$

 $M_U = 72.11 \le 75.4 = \phi M_v$

$$V_U = \frac{wl}{2} = \frac{(1.03klf)(23'8)^2}{2} = 12.19kips$$

$$V_U = 12.19 \le 79.2 = \phi V_n$$

Check Self Weight Assumption

Assumed
$$5psf > \frac{16lbs}{8'2} = 1.96$$
 \checkmark

Final Design: W12x16

$\frac{\text{Spacing:}}{\frac{18'5+23'8}{2}} = 21.04'$ Span = 24'5

- Deck and Concrete load = 37psf
- Superimposed dead load = 10psf
- Live load = 40psf

ive Load reduction

$$L = 0.4L_0$$
 -or-

$$L = L_0 * \left(0.25 + \frac{15}{\sqrt{K_{LL}A_T}}\right) = L_0 * \left(0.25 + \frac{15}{\sqrt{(24'5*21.04*2)}}\right) = 0.72L_0 - \text{controls}$$

Load Combinations:

1.4D = 75.6psf

1.2D+1.6L = 110.75psf - controls

$$W_u = 110.75psf * \frac{21.04'}{1000 \frac{lb}{klp}} = 2.33klf$$

$$M_U = \frac{wt^2}{8} = \frac{(2.33klf)(24r5)^2}{8} = 173.64ft \cdot kips \Rightarrow W14x30^2$$

 $M_U = 173.64 \le 177 = \phi M_{\odot}$

$$V_U = \frac{wl}{2} = \frac{(2.33klf)(24/5)^2}{2} = 28.45kips$$

$$V_{U} = 28.45 \le 112 = \phi V_{n} \checkmark$$

Check Self Weight Assumption:

Assumed
$$2psf > \frac{30lbs}{21.04t} = 1.43$$

Final Design: W14x30

$$A_T = 24'5 * \frac{23'8 + 18'5}{2} = 513.73 \, sf$$

- Deck and Concrete load = 37psf
- Superimposed dead load = 10psf
- Snow Load = 16pst
- Live load = 40psf
- Number of stories: 1 through 9 and the roof = 8 + roof Live Load reduction

$L = 0.5L_0$ - controls

$$L = L_0 * \left(0.25 + \frac{15}{\sqrt{K_{LL}A_T}}\right) = L_0 * \left(0.25 + \frac{15}{\sqrt{(8*4*513.73)}}\right) = 0.37L_0$$

Live Load = 40*0.5 = 20psf

Dead Load = 37+5+10+2 = 54psf

Load Combinations for the Roof

1.2D + 1.6L + 0.5S = 128.8psf - controls

128.8 psf * 513.73 sf = 66.168 lbs = 66.17 kips - for the roof

Load Combinations for the Floors:

1.2D + 1.6L = 90.4psf - controls

128.8 psf * 513.73 sf = 66.168 lbs = 66.17 kips - for the roof

 ${\scriptstyle (Number\ of\ Stories)*(90.4psf)*(513.73sf)} + 66.17 = Total\ Load$

ľ	Columns for Floors	Number of Stories	Total Load (Kips)	Column Design Size	Pult
	1-2	8	438		
	2-3	7	392		
	3-4	6	345		
	4-5	5	299	W10x49	522 kin
	5-6	4	252	WIUX49	532 kip
	6-7	3	206		
	7-8	2	160		
	8-9	1	113		

W10x33

Mechanical Breadth

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MAXIMUM CFM 6,000 6,000 ATURAL GAS 1' NPT 1' NPT PROPANE 1' NPT 1' NPT POWER SUPPLY 230/1/60/25* 230/1/60/25* V/PH/HZ/A SIMENSIONS x W x H (IN) 1,100,000 1,1			
BTU / HR MINIMUM 1,000,000 1,100,000 1,100,000 ATURAL GAS 1' NPT PROPANE 1' NPT 1' NPT POWER SUPPLY 230/1/60/25* 230/1/60/25* V/PH/HZ/A DIMENSIONS X W X H (IN) VEIGHT (LBS) 939 889	ECIFICATIONS	THP-1000	THP-1100
MINIMUM MAXIMUM CFM 6,000 6,000 ATURAL GAS 1' NPT 1' NPT PROPANE 1' NPT 1' NPT POWER SUPPLY 230/1/60/25* 230/1/60/25* V/PH/HZ/A DIMENSIONS X W X H (IN) VEIGHT (LBS) 939 889		3	
ATURAL GAS 1' NPT 1' NPT PROPANE 1' NPT 1' NPT POWER SUPPLY 230/1/60/25* 230/1/60/25* V/PH/HZ/A DIMENSIONS X W X H (IN) ZEIGHT (LBS) 939 889	BTU / HR MINIMUM MAXIMUM		
PROPANE 1' NPT 1' NPT POWER SUPPLY 230/1/60/25* 230/1/60/25* V/PH/HZ/A DIMENSIONS x W x H (IN) ZEIGHT (LBS) 939 889	CFM	6,000	6,000
POWER SUPPLY 230/1/60/25* 230/1/60/25* V/PH/HZ/A DIMENSIONS x W x H (IN) ZEIGHT (LBS) 939 889	ATURAL GAS	1" NPT	1" NPT
SUPPLY 230/1/60/25* 230/1/60/25* V/PH/HZ/A 0IMENSIONS 76x35x69 80x36x71 X W X H (IN) 939 889	PROPANE	1" NPT	1" NPT
76x35x69 80x36x71 x W x H (IN) /EIGHT (LBS) 939 889		230/1/60/25*	230/1/60/25*
	OIMENSIONS . x W x H (IN)	76x35x69	80x36x71
BASE CASTERS CASTERS	VEIGHT (LBS)	939	889
	BASE	CASTERS	CASTERS

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http://temp-air.com/heating/direct-fired-heaters-th/

Month	Temperature	Average
WIOIILII	Range (°F)	Temperature (°F)
November	26-41	33.5
December	12-27	19.5
January	8-24	16
February	13-29	21
March	24-41	32.5
April	37-58	47.5

ACI 306R-10 Guide to Cold Weather Concreting

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Keep the temperature of concrete as placed as close to the recommended minimum value as practicable.

Preparation before concrete is placed requires a temperature increase of the formwork, reinforcement, and other surfaces that will contact fresh concrete so the temperature of the freshly placed concrete will not decrease below the minimums as placed and maintained (Table 5.1). There are many techniques for warming formwork and embedded items, including heated enclosures, electric blankets, hydronic heating systems, or other acceptable means. Best practice indicates that all surfaces should be above the freezing temperature of water. However, take care to limit surface temperatures to no more than 10°F (5°C) greater or 15°F (8°C) less than that of the concrete to avoid inconsistent setting, rapid moisture loss, and plastic shrinkage cracking.

Table 5.1—Recommended concrete temperatures

		Section size, minimum dimension					
		< 12 in. (300 mm)	12 to 36 in. (300 to 900 mm)	36 to 72 in. (900 to 1800 mm)	> 72 in. (1800 mm)		
Line	Air temperature		Minimum concrete temperati	ure as placed and maintained			
1	_	55°F (13°C)	50°F (10°C)	45°F (7°C)	40°F (5°C)		
1		Minimum concrete temperature as mixed for indicated air temperature*					
2	Above 30°F (-1°C)	60°F (16°C)	55°F (13°C)	50°F (10°C)	45°F (7°C)		
3	0 to 30°F (-18 to -1°C)	65°F (18°C)	60°F (16°C)	55°F (13°C)	50°F (10°C)		
4	Below 0°F (-18°C)	70°F (21°C)	65°F (18°C)	60°F (16°C)	55°F (13°C)		
5	Maximum a		allowable gradual temperature drop in first 24 hours after end of pro		protection		
3	_	50°F (28°C)	40° (22°C)	30°F (17°C)	20°F (11°C)		
*							

*For colder weather, a greater margin in temperature is provided between concrete as mixed and required minimum temperature of fresh concrete in place

7.1—Protection methods

Protect concrete from freezing as soon as practicable after placement, consolidation, and finishing. This protection can be provided by concrete mixture acceleration, insulation, heat systems, enclosures, or a combination of these practices, and should be planned before placement. Accelerating the

Table 7.1—Length of protection period for concrete placed during cold weather

		Protection period at minimum tempera- ture indicated in Line 1 of Table 5.1, days*				
ine	Service condition	Normal-set concrete	Accelerated-set concrete			
1	No load, not exposed	2	1			
2	No load, exposed	3	2			
3	Partial load, exposed	6	4			
4	Full load	Refer to	Chapter 8			
\ dav	is a 24-hour period.					

Cost Analysis

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Category	Original Method	Structural Uplift Method	Cost Change (Original - Proposed)	
Construction Enclosure System	\$148,644.67	\$224,233.26	\$(75,588.59)	
Heating System	\$21,213.68	\$19,592.80	\$ 1,620.88	
Structure	\$1,205,195.30	\$1,834,730.94	\$(629,535.65)	
Equipment	\$144,864.00	\$264,680.39	\$(119,816.39)	
Exterior Enclosure	\$0	\$171,937.81	\$(171,937.81)	
General Conditions	\$0	\$(112,111.93)	\$112,111.93	
Total	\$1,519,917.64	\$2,556,279.89	\$(883,145.63)	

\$883,000 cost increase



33 Day Schedule Reduction

\$170,000 Increased Profit

Structural Lift Method Appendix

Years in Current Position

Years in Construction Industry

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